THE FUTURE OF DEEP FOUNDATIONS, TODAY



EXPANDER BODY [®] TECHNOLOGY SMART CELL[®] TECHNOLOGY



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EXPANDER BODY INTERNATIONAL INC.

TOOLS USED TO DESIGN

- SOIL INVESTIGATION: Conditions will differ between boreholes and therefore, the conditions assumed for the design can deviate from reality. (SPT, CPTu, DMT, PMT, Seismic, etc.)
- SOIL MODEL: The soil model used for the design could be less than suitable for predicting the actual pile response. (From simple formulas to *sophisticated* mathematical models)
- CONSTRUCTION METHODS: The methods may affect the soil conditions, so the models applied will become less adequate.
 (Drilled, CFA, FDP, Driven, Vibrated, Pushed, etc.)

MAIN CONCERN

Is it possible to truly predict the response of a pile?



WHAT REALLY HAPPENS

Scatter of assessments of the load-movement curves: capacity (?)



ADDITIONAL COMMENTS



Nq values for toe resistance of driven piles according to different authors.

WHO IS RIGHT? OR IS THE MODEL WRONG?

What if we could have RELIABLE information FOR EACH FOUNDATION UNIT?

WHAT IS NEEDED FOR THAT PURPOSE?

RELIABLE SYSTEMS, PROVIDING INFORMATION FOR EACH FOUNDATION UNIT

• HIGHER QUALTY CONTROL

• ECONOMICALLY COMPETITIVE SYSTEMS





WHAT IS THE EXPANDER BODY?



The Expander Body (EB), initially developed in Sweden, consists of a folded steel tube with a circular cross section.

It can be installed in the ground either as an enlarged pile base or as a soil anchor.

EXPANDER BODY PRIOR TO EXPANSION

















MONITORING OF EXPANSION PROCESS



The EB is inflated by high-pressure injection of grout. The result is an expanded, water-tight, high-strength, steel "balloon".

ABOUT THE EXPANDER BODY

- During expansion, lateral stresses around the EB are increased, creating a **high passive earth pressure.**
- The inflation pressure, which is measured, reflects the σ_h at the EB.



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EXPANDER BODY ACTS LIKE A PRESSUREMETER

- The Expander Body is similar to a large-scale pressuremeter and can be used as a soil testing device in each pile.
- By recording the injected grout volume and expansion pressure, the increase in soil stiffness and soil strength during inflation can be estimated. This information can be used to determine the toe capacity of a pile or an anchor.
- Based on a large number of pile and soil anchor applications, a semi-empirical design method was developed, which can be used to assess the toe response of piles and the pullout resistance of soil anchors.



INTERNATIONAL STANDARDS AND GUIDELINES



PRINCIPLE OF GROUT INJECTION IN EB

During the EB expansion, three typical injection phases can be identified:



CALIBRATION CURVE FOR EACH SIZE OF EB



This curve, for each size of EB, allows to know the **actual diameter for each volume** of grout injected.

HOW TO CREATE THE CALIBRATION CURVES





Typical grouting curve in medium dense sandy soil



Typical grouting curve in loose sandy soil. Note that the maximum pressure in loose sand is significantly lower than in a medium dense sand.



Leakage of grout during EB inflation before full expansion - or potential damage – can be detected by monitoring. The diameter used for the design of each pile can be adapted based on monitoring results.



Typical grouting curve in soft clay

INFLUENCE OF INSTALLATION PROCESS ON SHAPE OF INJECTION CURVE



- Blue curve (TP3) is from an EB installed **at base** of Full Displacement pile (FDP).
- Red curve (TP4) is from an EB installed at the toe of a drilled pile constructed under bentonite. The initial soil stiffness is softer because the drilled pile may have decompressed the surrounding soil and had debris at the pile toe. (The FDP leaves no debris and compresses the surrounding soil).

EXPANDER BODY TYPES

Model	Length prior to expansion	Length after expansion	Diameter of expanded body	Base Area	Surface Area	Maximum Volume
	m	m	m	m ²	m ²	m ³
EB 310	1.0	0.96	0.3	0.07	0.90	0.94
EB 410	1.0	0.86	0.4	0.13	1.10	0.11
E 610	1.0	0.76	0.6	0.28	1.43	0.21
EB 612	1.2	0.96	0.6	0.28	1.83	0.27
EB 615	1.5	1.26	0.6	0.28	2.38	0.36
EB 815	1.5	1.26	0.6	0.50	3.17	0.63
EB 820	2.0	1.76	0.8	0.50	4.42	0.88

- Typically, the **EB 600** series is used in **sandy soils**.
- The **EB 800** series was developed for **clayey soils** but now is also used in sandy soils.
- A new type, the EB 310, has been successfully used in stiff soils, cemented sands, and soft rocks.
- A new model, the **EB 1220, Ø1,20** m has also been introduced.

INSTALLATION OF THE EXPANDER BODY

As the cross section is small, several methods can be used for the installation.





INSTALLATION OF THE EXPANDER BODY



COMMON PILE TYPES IN COMBINATION WITH THE EB



Partial Displacement Auger pile with EB achieves higher shaft resistance than regular CFA and high toe resistances.



Full displacement auger pile (FDP) with EB achieves high shaft and toe resistances.

EXPANDER BODY PILES TYPICAL REPORT

PROJECT: TORRE CALACOTO					EBI PILES REPORT								INGENIERÍA Y CONSTRUCCIÓN				
CAP CODE	PILE	TIPO	SHAFT LENGTH (m)	SHAFT DIAMETER (m)	INJE PRESSURE (MPA)	CTION VOLUMEN (I)	INYECCION PRESION (kg/cm2)	DE FONDO VOLUME (I)	SHAFT RESISTANCE (kN)	SERVICE RE EBI FRICTION (KN)	ESISTANCE EBI TOE (KN)	TOTAL (kN)	EBI EQUIVALENT DIAMETER(mm)	AREA (sq m)	DESIGN SERVICE LOAD (kN)	MAX. LOAD PER CAP (kN)	REAL RESISTANCE PER CAP (kN)
CA3	1	1	14.60	0.50	2.44	280.20	3.59	70.00	764.45	559.67	1,129.79	2,453.92	788.71	0.49	1,500.00		
CA3	2	1	14.60	0.50	4.04	85.00			764.45	466.21	333.47	1,564.13	397.29	0.12	1,500.00		
																3,000.00	4,018.05
CB2	1	1	14.60	0.50	1.71	280.80			764.45	391.44	556.36	1,712.26	789.44	0.49	1,500.00		
CB2	2	1	14.60	0.50	5.39	280.00			764.45	1,236.85	1,755.78	3,757.08	788.46	0.49	1,500.00		
CB2	3	1	14.60	0.50	4.39	279.90			764.45	1,006.71	1,428.85	3,200.01	788.34	0.49	1,500.00		
CB2	4	1	14.60	0.50	2.73	281.50	4.04	70.30	764.45	628.14	1,270.81	2,663.40	790.29	0.49	1,500.00		
																6,000.00	11,332.76
CB3	1	1	14.60	0.50	2.45	280.40	4.33	70.20	764.45	562.60	1,203.26	2,530.32	788.95	0.49	1,500.00		
CB3	2	1	14.60	0.50	5.10	280.30			764.45	1,170.90	1,662.93	3,598.28	788.83	0.49	1,500.00		
CB3	3	1	14.60	0.50	3.23	280.60			764.45	740.65	1,052.36	2,557.46	789.19	0.49	1,500.00		
CB3	4	1	14.60	0.50	1.44	280.90	4.47	71.00	764.45	330.66	887.24	1,982.35	789.56	0.49	1,500.00		
CB3	5	1	14.60	0.50	2.28	280.40	4.25	70.50	764.45	524.05	1,141.04	2,429.55	788.95	0.49	1,500.00		
																7,500.00	13,097.96
CB4	1	1	14.60	0.50	1.41	280.10	4.89	70.70	764.45	323.82	916.16	2,004.44	788.58	0.49	1,500.00		
CB4	2	1	14.60	0.50	2.70	280.30			764.45	619.17	879.36	2,262.99	788.83	0.49	1,500.00		
CB4	3	1	14.60	0.50	3.92	110.00			764.45	529.47	442.74	1,736.66	464.44	0.17	1,500.00		
CB4	4	1	14.60	0.50	1.50	280.50	5.24	70.60	764.45	343.30	976.78	2,084.53	789.07	0.49	1,500.00		
CB4	5	1	14.60	0.50	2.59	281.70	5.93	71.20	764.45	595.91	1,401.62	2,761.98	790.53	0.49	1,500.00		
																7,500.00	10,850.60
CC2	1	1	14.60	0.50	1.51	280.40	4.12	70.10	764.45	345.77	875.68	1,985.91	788.95	0.49	1,500.00		
CC2	2	1	14.60	0.50	2.56	280.70	5.20	70.00	764.45	586.50	1,318.81	2,669.76	789.32	0.49	1,500.00		
CC2	3	1	14.60	0.50	3.22	130.00		-	764.45	482.31	446.82	1,693.58	514.56	0.21	1,500.00		
CC2	4	1	14.60	0.50	2.80	280.90	5.47	80.02	764.45	643.40	1,425.15	2,833.00	789.56	0.49	1,500.00		
CC2	5	1	14.60	0.50	1.54	281.40	2.91	70.00	764.45	353.66	774.73	1,892.84	790.17	0.49	1,500.00		
CC2	6	1	14.60	0.50	2.60	281.00	5.21	70.10	764.45	597.34	1,335.53	2,697.32	789.68	0.49	1,500.00		
																9,000.00	13,772.42
CD2	1	1	14.60	0.50	2.06	280.40	2.96	70.30	764.45	471.74	946.34	2,182.53	788.95	0.49	1,500.00		
CD2	2	1	14.60	0.50	2.56	280.90	3.22	70.80	764.45	588.52	1,137.13	2,490.11	789.56	0.49	1,500.00		
CD2	3	1	14.60	0.50	2.68	280.30	4.57	72.40	764.45	614.82	1,299.70	2,678.97	788.83	0.49	1,500.00		
CD2	4	1	14.60	0.50	2.97	280.90	3.42	70.00	764.45	681.75	1,288.32	2,734.53	789.56	0.49	1,500.00		
CD2	5	1	14.60	0.50	3.14	281.20	4.62	70.40	764.45	720.20	1,455.46	2,940.11	789.93	0.49	1,500.00		
CD2	6	1	14.60	0.50	4.34	145.00	2.58	70.20	764.45	694.10	928.17	2,386.72	550.05	0.24	1,500.00		
																9,000.00	15,412.96

POST- GROUTING OF EB

- During the expansion, the EB shortens, which induces tension in the shaft and lifts the EB base, which may soften the soil at the EB base.
- To counteract this effect, the soil below the EB is post-grouted, thus, increasing the pile toe stiffness and providing quality assurance.



POST- GROUTING OF EB



Similar to the expansion-grouting, **the post-grouting below and around the EB is monitored** to verify the improvement effect.

POST- GROUTING OF EB



EXHUMED EB WITH POST- GROUTING

RECENT ADVANCES

SELF BORING EXPANDER BODY:

- The micro pile is installed in one operation.
- It is a perfect solution for underpinning, working in limited access areas, or when only small equipment is available.



ANEXO 1

SOME LOADING TESTS CURVES

LOADING TESTS FOR MICROPILES WITH AND WITHOUT SELF BORING EB



MOVEMENT (mm)

LOADING TEST: DRILLED SHAFT IN SILTY SAND

SHAFT 450 mm IN DIAMETER, L=9 m. EB 612



PILE HEAD MOVEMENT A! (mm)

LOADING TESTS CURVES FOR PILES IN SANDS



ANCHORS

LOADING TEST: TIEBACKS IN COLLAPSABLE CLAY



GENERAL LIMITATIONS

1.- Typically, the higher service loads are limited to about 5000 kN for piles in very dense sands and 500 mm shaft.

2.- In sands, the minimum service load is around 250 kN, usually larger, therefore the EB is not competitive for lower loads in sands.

GENERAL ADVANTAGES

- 1.- Each EB performs a soil investigation with information about soil condition, soil type and stiffness. It can be used in all the types of soils. Also, the toe resistance increases dramatically, both in piles and anchors.
- **2.- Important economical savings**, between 15% and 60% compared with traditional systems. It can be installed with any system.
- **3.- Shorter construction time.** Up to 60% of time saving per resisted ton.
- 4.- High production. More than 20 EBs per shift, depending on the installation method.
- 5.- The carbon print per service ton is **much smaller** than in other systems.

BETTER QUALITY CONTROL, PRICE AND TIME.





PRE-LOAD, PRE-DEFORMATION CELLS FOR CAISSONS – SMART-CELL

After pile construction, the toe-box is expanded by injecting grout, which pushes the toe-box bottom into the soil at (below) the pile toe, compressing and stiffening it. This process includes measuring pressures and grout volumes.





Smart-cell before expansion

Smart-cell after expansion

TOE BOX SCHEME





TELL TALES

TOE BOX



The expansion of toe-box acts is similar to performing a bidirectional cell test because during the process, several parameters are recorded: pressure, volume, toe movement and head movement to provide data for post-construction—as is—analysis. N.B. <u>of every pile</u>!



Graph of measured pressure vs. injected grout volume

TYPICAL CURVE FORCE VS. TOE MOVEMENT.

"Force" is a measured pressure times the cross sectional area of the toe-box.

THE CHANGE OF STIFFNESS IS PRESENTED



TYPICAL CURVE FORCE VS. TOE MOVEMENT THE CHANGE OF STIFFNESS AND HEAD MOVEMENT IS PRESENTED



QUALITY SYSTEM

- Although the capacity of a pile should be defined prior to the installation of the toe box, during the expansion (injection of grout) the pressure, volume, and movements (toe and head) are measured, giving the resistance parameters of <u>each</u> pile/caisson.
- An additional important aspect is that the shape of the injected element is known.



DYMANIC LOAD TEST

- The following graphs show the results of two dynamic load tests (dlt) performed on two caissons 5m apart.
- The first has a toe box and second does not. The test employed a 15 t falling weight at 1.8 m free fall

DYNAMIC LOAD TEST

CAISSON Ø 1200 mm, L=22 m WITH TOE BOX



DYNAMIC LOAD TEST

CAISSON Ø 1200 mm, L=22 m WITHOUT TOE BOX



FORCE VS. MOVEMENT



BENEFITS



WHERE ARE WE NOW?



WHERE ARE WE PLANNING TO BE IN 2020?





THANK YOU TAIWAN!

